[redacted]

DESIGN BASIS

D1. STRUCTURAL WORK HAS BEEN DESIGNED FOR THE FOLLOWING:

PERMANENT DEAD LOAD OF THE STRUCTURE, AND FINISHES	NOMINATED ON ARCHITECTURAL DRAWINGS
LIVE LOADS TO AS/NZS1170.1	0.25kPa ON ROOF
BUILDING DESIGN WORKING LIFE	50 YEARS
BUILDING IMPORTANCE LEVEL	3
WIND LOAD TO AS/NZS1170.2:	
REGION	A5
AVERAGE RECURRENCE INTERVAL	1000 YEARS
ULTIMATE REGIONAL WIND SPEED	46 m/s
SERVICEABILITY REGIONAL WIND SPEED	37 m/s
DIRECTION MULTIPLIER	1.0
TERRAIN CATEGORY	2.0
TERRAIN/HEIGHT MULTIPLIER	1.0
SHIELDING MULTIPLIER	1.0
TOPOGRAPHIC MULTIPLIER	1.0

DEAD LOAD	
ROOF SDL	0.25kPa
MEZZANINE SDL	0.5kPa
OFFICE SDL	0.25kPa
GENERAL FLOOR SDL	0.25kPa
LIVE LOAD	
ROOF LL	0.25kPa
MEZZANINE LL	3kPa
OFFICE LL	2kPa
GENERAL FLOOR LL	5kPa
MEZZANINE IS EVALUATED FOR THE FOLLOWING EQUIPMENT LOAD:	
CARDIO EQUIPMENT	PER UNIT KG
TREADMILLS	200.00 kg
CROSS TRAINERS	208.00 kg
UPRIGHT BIKE	80.00 kg
RECUMBENT BIKE	98.00 kg
POWERMILL	215.00 kg
SPIN BIKE	42.00 kg
GENERAL FLOOR IS EVALUATED FOR 2 TONS FOR LIFT	

NOTE: LOADS FROM MECHANICAL EQUIPMENT OR SERVICES HAVE NOT BEEN ALLOWED FOR UNO. LOADS SHALL BE PROVIDED TO FOR DESIGN AND VERIFICATION PRIOR TO CONSTRUCTION/FABRICATION BY THE BUILDER/ARCHITECT.

D2. FOOTING DESIGN BASIS:
(a) ALLOWABLE BEARING PRESSURE (STRIP FOOTING) = 100kPa
(b) STRUCTURAL DESIGN BASED ON ASSUMED SOIL CONDITIONS WHICH SHALL BE CONFIRMED PRIOR TO BUILDING PERMIT. CLASS M SITE.

GENERAL

G1. THESE DRAWINGS AND NOTES SHALL BE READ IN CONJUNCTION WITH ALL ARCHITECTURAL AND OTHER CONSULTANTS DRAWINGS, REPORTS, SPECIFICATIONS, AND ANY OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED DURING THE COURSE OF CONSTRUCTION. ALL DISCREPANCIES SHALL BE REFERRED TO THE ENGINEER FOR CLARIFICATION OR DECISION BEFORE PROCEEDING WITH THE WORK.

G2. THE BUILDER SHALL ENSURE ALL WORKS ARE CONSTRUCTED IN ACCORDANCE WITH ENGINEERING DOCUMENTATION AND INSPECTED BY THE PROJECT BUILDING SURVEYOR IN ACCORDANCE WITH THE RELEVANT AUTHORITY FOR COMPLIANCE WITH THE DESIGN DRAWINGS AND SPECIFICATIONS. A MINIMUM OF 48HRS NOTICE SHALL BE GIVEN FOR INSPECTIONS. THE BUILDER SHALL ALLOW FOR COSTS FOR ANY ENGINEERING WORKS DURING CONSTRUCTION TO CARRY OUT INSPECTIONS AND OTHER CONSTRUCTION PHASE SERVICES AS INSTRUCTED.

G3. WORKS SHALL NOT COMMENCE UNTIL APPROVED BY THE RELEVANT AUTHORITIES, NECESSARY PERMITS ARE OBTAINED, AND DRAWINGS STATUS IS DOCUMENTED AS ISSUED 'FOR CONSTRUCTION'.

G4. DIMENSIONS SHALL NOT BE OBTAINED FROM THE STRUCTURAL DRAWINGS BY WAY OF SCALING (OR DETERMINING MEASUREMENTS FROM THE ELECTRONIC DRAWINGS). ALL STRUCTURAL MEMBER SIZES SHOWS ON THESE DRAWINGS SHALL TAKE PRECEDENCE OVER THOSE ON THE ARCHITECT'S DRAWINGS. DISCREPANCIES SHALL BE REFERRED TO THE ARCHITECT. THE BUILDER SHALL VERIFY ALL DIMENSIONS ON SITE AND BE RESPONSIBLE FOR THE SETTING OUT OF THE BUILDING.

G5. THESE DRAWINGS DO NOT DETAIL TEMPORARY WORKS. TEMPORARY WORKS ARE THE RESPONSIBILITY OF THE BUILDER, INCLUDING PROPPING, BRACING, SHORING AND ANY OTHER REQUIREMENTS NECESSARY TO MAINTAIN THE STRUCTURE, OR ANY PART OF IT. IN A STABLE CONDITION DURING CONSTRUCTION. IF THE BUILDER IS IN ANY DOUBT AS TO HOW TO ACHIEVE THIS, HE SHALL OBTAIN ADVICE FROM APPROPRIATELY QUALIFIED AND EXPERIENCED PERSONS AND SHALL AS A MINIMUM BE A REGISTERED ENGINEER WITH THE VICTORIAN BUILDING AUTHORITY. UNLESS STATED OTHERWISE, TEMPORARY WORKS SHALL BE THE BUILDERS RESPONSIBILITY.

G6. THE BUILDER SHALL LOCATE ALL EXISTING AND PROPOSED SERVICES AND EASEMENTS, ON AND ADJACENT TO THE SITE. THE APPROVAL OF THE RELEVANT STATUTORY AUTHORITY AND THE ENGINEER SHALL BE OBTAINED BEFORE BUILDING ON, OVER OR PART OF THE STRUCTURE IS FOUNDED WITHIN A 45 DEGREE ANGLE OF REPOSE OF ANY SERVICES OR EASEMENTS.

G7. THE WRITTEN CONSENT OF ADJOINING PROPERTY OWNER SHALL BE OBTAINED BEFORE INSTALLATION OF UNDERPINNING WORK, EXCAVATIONS AND/OR INSTALLATION OF DRAINAGE LINES OR ANY OTHER WORK BEYOND THE PROPERTY BOUNDARY, OR IF ANY EXCAVATION IN UNDERTAKEN BELOW A 45 DEGREE ANGLE OF REPOSE OF ANY NEIGHBOURING BUILDING FOUNDATION.

G8. ALL WATERPROOFING SHALL BE SPECIFIED BY THE ARCHITECT. WATERPROOFING IS NOT INCLUDED WITHIN THIS SPECIFICATION AND/OR DRAWINGS. THE BUILDER SHALL SEEK ADVISE FROM THE ARCHITECT FOR PROJECT SPECIFIC WATERPROOFING REQUIREMENTS.

G9. SUBMIT DETAILS OF PROPOSED CHANGES TO SCOPE, WORK METHOD OR MATERIALS ETC FOR APPROVAL BEFORE PROCEEDING.

G10. WORKMANSHIP AND MATERIALS TO COMPLETE WITH REQUIREMENTS OF AUSTRALIAN STANDARDS, NATIONAL CONSTRUCTION CODE (NCC) AND BY-LAWS.

G11. ENGINEER REFERS TO AN EMPLOYEE OF , UNO = UNLESS NOTED OTHERWISE.

FOUNDATIONS

F1. FOUNDATION MATERIAL SHALL BE APPROVED BY THE CONSULTING GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF CONCRETE.

F2. DEEPEN FOOTING AND PROVIDE BLINDING CONCRETE BELOW AS REQUIRED TO ACHIEVE THE REQUIRED SOIL BEARING CAPACITY, AND/OR NEAR EXISTING OR PROPOSED SERVICES, EXCAVATIONS, BATTERS ETC, SO INFLUENCE LINE (AT 45 DEGREES FROM THE HORIZONTAL) FROM FOOTING IS BELOW ADJACENT EXCAVATION.

F3. LAG ALL SERVICES PENETRATING FOOTINGS WITH 40mm THICK CLOSED CELL POLYETHYLENE LAGGING.

F4. BASES OF ALL FOOTINGS SHALL BE CLEANED OF ALL LOOSE MATERIAL PRIOR TO POURING OF CONCRETE. PROVIDE 50mm BLINDING CONCRETE LAYER BELOW ALL FOOTINGS.

F5. ALL WORKS TO BE IN ACCORDANCE WITH AS2870.

GROUND SLABS

F7. ALL VEGETATION SHALL BE STRIPPED TO A MINIMUM DEPTH OF 150mm. ANY SOFT SPOTS OR DELETERIOUS MATERIAL SHALL BE REMOVED AND REPLACED WITH APPROVED GRANULAR FILLING COMPACTED TO 100% AS. STANDARD COMPACTION. MINOR FILLING (800 MAXIMUM) SHALL BE PROVIDED WHERE REQUIRED TO BRING SUB GRADE TO REQUIRED LEVEL IN ACCORDANCE WITH LIMITS STATED IN AS 2870 AND BCA. FILLING SHALL BE APPROVED GRANULAR MATERIAL PLACED IN 150mm AND COMPACTED TO 100% AS. STANDARD COMPACTION.

F8. A 0.2mm VAPOUR BARRIER SHALL BE USED, LAPPED A MINIMUM OF 200mm AT JOINTS AND TAPED AROUND SERVICES FITTINGS WITH ADHESIVE TAPE NOT INFERIOR TO DOUBLE SIDED BUTYL ADHESIVE TAPE. THE VAPOUR BARRIER SHALL BE PLACED ON A50mm MINIMUM SAND BED OR SIMILAR APPROVED MATERIAL. PROTECT MEMBRANE FROM DAMAGE.

F9. FOUNDATION MAINTENANCE TO BE IN ACCORDANCE WITH CSIRO BUILDING TECHNOLOGY FILE 18 FOUNDATION MAINTENANCE AND FOOTING PERFORMANCE: A HOMEOWNERS GUIDE'.

F10. TRENCH MESH IN BEAMS SHALL BE OVERLAPPED BY THE WIDTH OF FABRIC AT 'T' AND 'L' INTERSECTIONS AND SPLICED WITH A LAP OF 500mm. RANDOM LAP N12 BARS BY 500mm STAGGERED. THE OUTER BAR AT L'INTERSECTION MUST BE BENT AND CONTINUED FOR 500mm AROUND THE CORNER.

F11. SLAB FABRIC TO BE LAPPED SUCH THAT THE TWO OUTERMOST TRANSVERSE WIRE OF ONE SHEET OF MESH OVERLAP THE TWO OUTERMOST TRANSVERSE WIRES OF THE SHEET BEING LAPPED BY A MINIMUM OF 25mm AND BE SUPPORTED ON BAR CHAIRS AT 800mm MAXIMUM CENTRES

F12. FOUND ALL SLAB PANELS ON NATURAL MATERIAL OF 50kPa MINIMUM ALLOWABLE BEARING CAPACITY. FOUND ALL GROUND BEAMS ON NATURAL MATERIAL OF 100kPa ALLOWABLE BEARING CAPACITY.

CONCRETE

C1. ALL CONCRETE WORKS TESTING AND MATERIALS SHALL BE IN ACCORDANCE WITH AS3600 AND OTHER RELEVANT AUSTRALIAN STANDARDS UNO.

C2. COVER TO REINFORCEMENT (IN mm) AND CONCRETE GRADES SHALL BE IN ACCORDANCE WITH THE FOLLOWING TABLE UNO:

ELEMENT	GRADE	COVER (INTERNAL)	COVER (EXTERNAL/AGAINST GROUND)
FOOTINGS/PIERS	N25	-	60
SLAB	N32	25	40
WALLS	N40	40	50

NOTES:

(a) COVER FOR ELEMENTS POURED AGAINST GROUND NOTED ABOVE IS APPLICABLE ONLY WHEN CAST AGAINST A DAMP PROOF MEMBRANE. ADD 20MM COVER FOR CAST DIRECTLY AGAINST GROUND.
(b) INTERNAL IS DEFINED AS THE SURFACE OF A CONCRETE ELEMENT EXPOSED TO AN INTERNAL, WEATHERPROOF AND FULLY ENCLOSED NON-INDUSTRIAL ENVIRONMENT.
(c) EXTERNAL IS DEFINED AS THE SURFACE OF A CONCRETE ELEMENT EXPOSED TO THE WEATHER AND EXTERNAL ATMOSPHERE, NON-INDUSTRIAL
(d) COVER IS DISTANCE FROM FACE OF THE CONCRETE ELEMENT, AND THE OUTER FACE OF THE REINFORCING.
(e) EXTERNAL CONCRETE ELEMENTS SHALL HAVE A MINIMUM PORTLAND CEMENT CONTENT OF 330kg/m3, A MAXIMUM WATER/CEMENT RATION OF 0.5, AND CHLORIDE CONTENT RESTRICTED AS PER CLAUSE 4.9 OF AS3600.

C3. ALL CONCRETE SHALL BE CURED TO AS3600 FOR A MINIMUM OF 7 DAYS WITHIN 2 HOURS OF POURING, CURING SHALL BE BY ONE OR MORE OF THE FOLLOWING METHODS:

(a) CONTINUOUS SATURATION WITH POTABLE WATER
(b) APPLICATION OF AN APPROVED PROPRIETARY CURING COMPOUND COMPLYING WITH AS399, APPLIED IN ACCORDANCE WITH THE MANUFACTURES REQUIREMENTS, AND SUITABLE FOR APPLIED FINISHES
(c) RETENTION OF FORMWORK
(d) AN ABSORBENT COVER KEPT CONTINUOUSLY WET
(e) STEAM CURING
(f) AN IMPERMEABLE MEMBRANE

C4. DO NOT PLACE CONCRETE WHEN AMBIENT TEMPERATURE, REINFORCEMENT OR FORMWORK IS GREATER THAN 32 DEGREES. PROTECT CONCRETE FROM PREMATURE DRYING, PARTICULARLY IN HOT, WINDY OR DRY CONDITIONS.

C5. PROVIDE 25MM CHAMFER TO ALL EXTERNAL CORNERS OF WALLS, CORBELS, HOBS/UPSTANDS, BEAM SOFFITS etc.

C6. CONCRETE FINISHES TO ARCHITECTS DETAILS.

C7. SAW CUTS SHAL BE MADE WITHIN 24HRS OF CONCRETE POUR.

C8. ALL PENETRATIONS, CHASES, AND HOLES SHALL NOT BE MADE IN CONCRETE MEMBERS WITHOUT PRIOR APPROVAL.

C9. CONDUITS/PIPES SHALL BE PLACED WITHIN THE MIDDLE THIRD OF THE SLAB DEPTH, BETWEEN THE TWO REINFORCEMENT LAYERS AND AT A MINIMUM SPACING OF NOT LESS THAN 3 DIAMETERS. PROVIDE MINIMUM 50mm COVER TO REINFORCING. LOCATION OF CONDUITS TO BE APPROVED BY THE ENGINEER.

REINFORCEMENTS

R1. THE REINFORCEMENT SYMBOLS ON DRAWINGS FOR GRADE AND TYPE OF REINFORCEMENT ARE:

- N: NORMAL DUCTILITY CLASS HOT ROLLED 500N DEFORMED BARS WITH fsy = 500 MPa.
- R: NORMAL DUCTILITY CLASS 250N PLAIN ROUND BARS WITH fsy = 250 MPa.
- L: LOW DUCTILITY CLASS HARD DRAWN 500L WIRE REINFORCING MESH WITH fsy = 500MPa.
- TM: LOW DUCTILITY CLASS HARD DRAWN 500L TRENCH MESH
- SL: LOW DUCTILITY CLASS HARD DRAWN 500L SQUARE MESH
- RL: LOW DUCTILITY CLASS HARD DRAWN 500L RECTANGULAR MESH

R2. ALL REINFORCEMENT BARS SHALL BE TO AS/NZS 4671.

R3. PROVIDE STANDARD COGS AND HOOKS TO AS3600, ALL BENDING AND RE-BENDING SHALL BE CARRIED OUT TO AS3600 AND AS/NZS 4671.

R4. REINFORCEMENT IS SHOWN DIAGRAMMATICALLY. IT IS NOT NECESSARILY SHOWN IN TRUE PROJECTION.

R5. LAPS AND SPLICES OF REINFORCEMENT SHALL BE MADE AT THE LOCATIONS SHOWN ON THE DRAWINGS. ADDITIONAL SPLICES REQUIRE THE WRITTEN APPROVAL OF THE ENGINEER. LAP SPLICES UNO SHALL BE AS FOLLOWS:

BAR SIZE	MINIMUM LAP LENGTH BEAMS/SLABS/WALLS		MINIMUM LAP LENGTH COLUMNS
	25MPa	>=32MPa	>=40MPa
N10	600	500	-
N12	800	650	-
N16	1200	1000	-
N20	1500	1300	650
N24	1900	1600	800
N28	2200	1900	900
N32	2700	2300	1050
N36	3200	2700	1150

R6. PROVIDE MINIMUM LAPS TO CROSS WIRES OF REINFORCING MESH, SO TWO OUTERMOST WIRES OF ONE SHEET OVERLAP TWO OUTERMOST LAYERS OF ADJACENT SHEET BY AT LEAST 25mm.

R7. CHAIR ALL REINFORCEMENT AT 800 MAXIMUM CENTRES, AND TIE TO KEEP IN POSITION AND TO MAINTAIN CONCRETE COVERS.

R8. STEEL CHAIRS SHALL ONLY BE PLASTIC TIPPED, AND ONLY USED FOR INTERNAL APPLICATIONS. FULLY PLASTIC CHAIRS SHALL BE USED FOR EXTERNAL APPLICATIONS, AND WHERE REINFORCEMENT IS GROUND SUPPORTED AND CHAIRS SHALL BE SUPPORTED ON PLATES.

STEEL WORK

S1. SUBMIT SHOP DRAWINGS TO THE ENGINEER FOR REVIEW AND APPROVAL BEFORE COMMENCING FABRICATION. APPROVAL WILL NOT COVER DIMENSIONS OF MEMBERS AND LAYOUT. ALLOW FOR 5 BUSINESS DAYS FOR ENGINEER REVIEW. ALL DETAILS, GAUGE LINES, ETC. WHERE NOT SPECIFICALLY SHOWN SHALL BE IN ACCORDANCE WITH AISC DESIGN CAPACITY TABLES FOR STRUCTURAL STEEL AND AISC STANDARDISED STRUCTURAL CONNECTIONS.

S2. MINIMUM CONNECTION UNO SHALL BE 10mm THICK CLEAT PLATES. BOLTS TO BE M20-8.8/S IN 22mm DIAMETER HOLES. PROVIDE A MINIMUM OF TWO BOLTS PER CONNECTION.

S3. FIXINGS FOR ARCHITECTURAL ELEMENTS NOT SHOWN. FABRICATOR SHALL PROVIDE ALL FIXINGS WITHOUT WEAKENING THE STRUCTURAL MEMBER IN ANY WAY.

S4. CAMBER SHALL BE PROVIDED TO ALL ROOF BEAMS, TRUSSES, AND PORTALS ETC. AT 5 PER 2000 OF SPAN UNLESS OTHERWISE NOTED, FOR ALL MEMBERS SPANNING IN EXCESS OF 6m.

S5. WORKMANSHIP, FABRICATION AND MATERIALS TO COMPLY WITH AS4100, AS/NZS4600, AS/NZS 1554, AS/NZS5131 AND AS4673 FOR STAINLESS STEEL

S6. UNLESS NOTED OTHERWISE, ALL MATERIAL TO BE:

- GRADE 250 HOT ROLLED PLATES, FLATS, ANGLES, 100 x 100 ANGLE OR 125 x 75 ANGLE AND SMALLER;
- GRADE 300PLUS UB, UC, PFC AND LARGER ANGLES -
- GRADE 300 WB, WC; -
- GRADE 350 RHS, CHS.

S7. PROVIDE STEELWORK REFERENCE IN THESE DRAWINGS IN ACCORDANCE WITH AS1163, AS1397, AS1443 AS/NZS1594, AS/NZS3678, AS/NZS3679.

S8. PROVIDE 3mm CAP PLATES WELDED TO END OF ALL HOLLOW SECTIONS UNO.

S9. BOLT HOLE SIZE SHALL BE AS FOLLOWS UNO:

- BOLT DIAMETER PLUS 2mm FOR STEEL TO STEEL CONNECTIONS
- BOLT DIAMETER PLUS 4MM FOR STEEL TO CONCRETE CONNECTIONS
- BOLT DIAMETER PLUS 4MM FOR HOLDING DOWN BOLTS UP TO M20 BOLT DIAMETER PLUS 6mm
- FOR HOLDING DOWN BOLTS M24 OR GREATER

BOLTING/CONNECTIONS

S10. BOLTING PROCEDURE IS DESIGNATED AS FOLLOWS:

- 4.6/S REFERS TO COMMERCIAL BOLTS OF STRENGTH GRADE 4.6 TO AS/NZS 1111, SNUG TIGHTENED
- 8.8/S REFERS TO HIGH STRENGTH BOLTS OF STRENGTH GRADE 8.8 TO AS/NZS 1252, SNUG TIGHTENED
- 8.8/TF REFERS TO HIGH STRENGTH BOLTS OF STRENGTH GRADE 8.8 TO AS/NZS 1252 FULLY TENSIONED TO AS1511, DESIGNED AS FRICTION TYPE
- 8.8/TB REFERS TO HIGH STRENGTH BOLTS OF STRENGTH GRADE 8.8 TO AS/NZS 1252 FULLY TENSIONED TO AS 1511, DESIGNED AS A BEARING TYPE JOINT

S11. LOAD INDICATING WASHERS SHALL BE USED TO VERIFY TIGHTENING OF BOLTS IN TF AND TB CONNECTIONS. A HARDENED WASHER SHALL BE USED UNDER THE BOLT HEAD OR NUT, WHICHEVER IS ROTATED. FULLY TENSIONED BOLTS SHALL NOT BE RE-USED. WELDING OF CAPTIVE NUTS TO STEELWORK SHALL BE GRADE 4.6S, CLASS 5 NUTS. THE ELECTRODES USED SHALL BE COMPATIBLE WITH THE CHEMISTRY OF THE STEEL INVOLVED (MEMBER OR CONNECTION COMPONENT AND NUT). ALL SUCH WELDS SHALL HAVE 100% VISUAL INSPECTION. GRADE 4.6S BOLTS TO BE USED.

S12. FACES OF FRICTION GRIP CONNECTIONS SHALL NOT BE PAINTED OR GALVANISED.

S13. ALL BOLTS SHALL BE HOT DIPPED GALVANISED AND OF SUCH LENGTH THAT AT LEAST ONE FULL THREAD IS EXPOSED BEYOND THE NUT AFTER THE NUT HAS BEEN TIGHTENED. MINIMUM ONE WASHER SHALL BE USED UNDER THE NUT IN ALL SITUATIONS. IF TIGHTENING IS CARRIED OUT AT THE HEAD, AN ADDITIONAL WASHER SHALL BE USED UNDER THE HEAD. FOR SLOTTED HOLES, SHORTER THAN THE LESSER OF 1.33 TIMES THE BOLT DIAMETER OR (BOLT DIAMETER + 10mm) AND NOT WIDER THAN THE BOLT DIAMETER PLUS 2mm, USE HARDENED WASHER UNDER THE NUT AND BOLT HEAD.

WELDING

S14. ALL WELDING TO BE IN ACCORDANCE WITH AS/NZS 1554 AND AS/NZS 1553.

S15. UNO WELDS TO BE 6mm CONTINUOUS FILLETS, CATEGORY SP

S16. ALL WELDS TO BE SHOP WELDED UNO, CATEGORY SP, ELECTRODES SHALL BE E49XX

S17. EXTENT OF WELD TESTING SHALL BE:

- VISUAL SCANNING: 100% OF WELDS
- VISUAL EXAMINATION: 100% OF BUTT WELDS, AND 50% OF OTHERS
- RADIOGRAPHIC AND ULTRASONIC: 10% OF BUT WELDS, 5% OF OTHER

DURABILITY/COATINGS

S18. ALL INTERNAL STEELWORK, NOT EXPOSED TO WEATHER OR EXTERNAL ATMOSPHERE SHALL BE GRIT BLASTED TO CLASS 2.5, AND PAINTED WITH BLUE ZINC PHOSPHATE 75um DRY FILM THICKNESS, UNO BY THE ARCHITECTURAL SPECIFICATION. ALL COATING WORKS TO BE IN ACCORDANCE WITH AS/NZZS2312.

S19. ALL EXTERNAL STEELWORK SHALL BE HOT DIP GALVANISED TO AS4680, WITH MINIMUM COATING THICKNESS OF 85 MICRONS.

PROFILED STEEL DECKING

PD1. MINIMUM FIXINGS
DECKING SHALL BE FIXED IN ACCORDANCE WITH DECKING MANUFACTURER'S RECOMMENDATIONS, AND THE MINIMUM OF:
(a) 12mm DIAMETER PUDDLE WELD LOCATED AT 400 CRS.
(b) 4 mm DIAMETER POWER ACTIVATED DRIVEPINS LOCATED AS PER MANUFACTURER'S RECOMMENDATIONS.
(c) 19mm DIAMETER SHEAR STUDS WELDED THROUGH DECKING AT 400 CRS.
(d) 50mm SEAT WIDTH ON CONCRETE SURFACES.

PD2. FIXING LOCATIONS
FIXINGS SHALL BE WELDED IN MIDDLE OF THE DECK RIB WHERE POSSIBLE. IF WELDED OFF CENTRE THE SIDE ON WHICH THE FIXING IS WELDED SHALL BE ALTERNATED FROM RIB TO RIB.

PD3. DECK PROPERTIES
FLOOR DECK SHALL BE GRADE G550 1.0mm BMT PRESSED METAL RIBBED PROFILE WITH MINIMUM Z350 GALVANISED FINISH TO AS 1397 UNO.

PD4. CONTINUOUS SPANS
THE DECK SHALL BE LAID CONTINUOUS OVER MINIMUM 4 SPANS WHERE POSSIBLE.

PD5. PENETRATIONS
FLOOR PENETRATIONS SHALL BE FORMED IN THE CONCRETE AND THE DECK CUT ONLY AFTER THE CONCRETE HAS ATTAINED MINIMUM 15 MPa STRENGTH.

PD6. COLUMNS
WHERE THE METAL DECK NEEDS TO BE CUT FOR COLUMNS PASSING THROUGH THE FLOOR, PROVIDE 75x50x1.0 GALVANISED ANGLES (OR ALTERNATIVE ENGINEER APPROVED SOLUTION) TO SUPPORT THE CUT DESK EDGES.

PD7. SERVICES CONDUITS
FLOOR SERVICES CONDUITS SHALL BE KEPT CLEAR MINIMUM 100mm FROM ANY SHEAR STUD.

PD8. EDGE FINISHING
EDGES AND ENDS OF PROFILED STEEL DECKING SHALL BE ENCASED IN CONCRETE, OR FINISHED WITH A 1.0mm MINIMUM THICKNESS PROFILED GALVANISED STEEL EDGE.

PD9. PROPPING
PROPPING SHALL BE IN ACCORDANCE WITH MANUFACTURER'S REQUIREMENTS OR AS SPECIFIED ON THE DRAWINGS.

PRECAST CONCRETE

PC1. WORKMANSHIP
ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS 3600 EXCEPT AS VARIED BY THE CONTRACT DOCUMENTS.

PC2. PRECAST PANELS
PRECAST CONCRETE IS TO BE GENERALLY IN ACCORDANCE WITH AS 3850 "TILT-UP" CONCRETE CONSTRUCTION.

PC3. DIMENSIONS
FOR LOCATIONS AND NON-STRUCTURAL DIMENSIONS OF PRECAST PANELS REFER TO ARCHITECTURAL DRAWINGS.

PC4. FORMED SURFACE
PANELS ARE TO CONSIST OF OFF-FORM CONCRETE POURED IN APPROVED MOULDS AS SPECIFIED BY ARCHITECT.

PC5. CONCRETE STRENGTH
THE CONCRETE IN THE PANELS IS TO HAVE A CHARACTERISTIC STRENGTH (f'c) AS SPECIFIED ON THE DRAWINGS. THE CONCRETE STRENGTH AT REMOVAL FROM MOULDS IS TO BE A MINIMUM OF 20MPa. ALL PANELS ARE TO BE CONSTRUCTED FROM NORMAL WEIGHT CONCRETE.

PC6. CORBELS
ALL CORBELS TO THE PRECAST PANELS TO BE POURED MONOLITHICALLY WITH THE PANEL.

PC7. CONCRETE MIX
WATER/CEMENT RATIO NOT EXCEED 0.45 SLUMP NOT TO EXCEED 50mm.

PC8. CHEMICAL CONTROLS
CALCIUM CHLORIDE WILL UNDER NO CIRCUMSTANCES BE PERMITTED, AND OTHER ADMIXTURES WILL ONLY BE PERMITTED WITH THE WRITTEN CONSENT OF THE CONTRACTOR.

PC9. CLEANING
ACID ETCHING WILL UNDER NO CIRCUMSTANCES BE PERMITTED. ACID CLEANING WITH DILUTE HYDROCHLORIC ACID OF MAXIMUM CONCENTRATION OF 20 TO 1 MAY BE PERMITTED UNDER CONTROLLED CONDITIONS IF THE CHLORIDE ION PENETRATION AND CONCENTRATION IS WITHIN ACCEPTABLE LIMITS. HIGH PRESSURE WATER JETS ARE TO BE USED TO REMOVE ALL TRACES OF ACID FROM THE SURFACE. FULL DETAILS OF PROPOSED CLEANING PROCEDURE IS TO BE SUBMITTED TO THE CONTRACTOR FOR WRITTEN APPROVAL PRIOR TO CLEANING.

PC10. CHLORIDE ION CONTENT
THE MAXIMUM CHLORIDE ION CONTENT BY MASS OF CEMENT IS NOT TO EXCEED 0.1%. CHEMICAL TEST RESULTS ON TRIAL PANELS TREATED IN THE INTENDED MANNER SHALL BE SUBMITTED TO THE CONTRACTOR AND APPROVAL GAINED PRIOR TO MANUFACTURE.

PC11. NOT PERMITTED
VENEERED CONSTRUCTION AND/OR USE OF DIFFERENT CONCRETE MIX AND/OR USE OF DIFFERENT CONCRETE SUPPLIERS IS NOT PERMITTED.

PC12. APPROVAL REQUIRED
DETAILS OF ALL SURFACE TREATMENTS ARE TO BE SUBMITTED TO THE CONTRACTOR FOR WRITTEN APPROVAL PRIOR TO MANUFACTURER.

PC13. REINFORCEMENT COVER
CONCRETE COVER SHALL BE AS NOTED ON THE DRAWINGS MINIMUM COVER FOR EXPOSED FACE 35 mm AND INTERNAL FACE 25mm, WHICHEVER IS THE GREATER TOLERANCE +5mm, -0mm UNO.

PC14. LIFTING
LIFTING FERRULES ARE TO BE DESIGNED AND DETAILED BY THE PRECAST MANUFACTURER TO SUIT THE METHOD OF PRECAST MANUFACTURE, TRANSPORT AND ERECTION. TYPICAL LIFTING FERRULES ARE TO BE AT LEAST 24mm DIAMETER, HOT DIPPED GALVANISED STEEL FERRULES FITTED WITH A "J" BAR ANCHOR CAPABLE OF DEVELOPING AT LEAST THE FULL DESIGN CAPACITY OF THE BOLT. LIFTING FERRULES ARE NOT SHOWN ON DESIGN DRAWINGS. LIFTING FIXINGS SHALL NOT BE LOCATED IN THE FACE WHICH IS EXPOSED TO VIEW IN THE FINAL CONDITION AND AFTER USE SHALL BE PROTECTED TO AVOID CORROSION AND STAINING. FERRULES THAT WOULD BE EXPOSED ON COMPLETION OF ERECTION ARE TO BE SET 25mm BELOW FINISHED LEVEL AND THE RECESS TO BE GROUTED ON COMPLETION.

PC15. CONNECTIONS
ALL CONNECTION MEMBERS AND FIXINGS ARE TO BE HOT DIPPED GALVANISED.

PC16. JOINT SEALANT
CLOSED CELL POLYETHYLENE FOAM ROD COMPRESSION SEALS (25% MIN. COMPRESSION) OR APPROVED EQUIVALENT TO BE USED AS SHOWN ON DETAILS TO CONTAIN IN-SITU GROUT AND TO EXCLUDE CONCRETE SCREED IN JOINTS BETWEEN PANEL AND IN-SITU FLOOR. FOR DETAILS OF FIRE PROTECTION AND WEATHER PROOFING OF JOINTS REFER TO ARCHITECT'S DRAWINGS.

PC17. SHOP DRAWINGS
PRECAST MANUFACTURER TO PROVIDE A (pdf) FILE OF EACH FULLY DETAILED SHOP DRAWING INCLUDING DETAILS OF ALL INSERTS, FIXINGS, ANCHORS, BOLTS, FERRULES, PENETRATIONS, LIFTING DEVICES, ETC TO AND TO THE CONTRACTOR FOR APPROVAL.

PC18. ERECTION
THE PRECAST CONTRACTOR IS TO PROVIDE TEMPORARY SUPPORTS DURING THE ERECTION OF THE PRECAST PANELS AS REQUIRED TO ENSURE STABILITY OF THE PRECAST UNITS UNTIL THE PERMANENT FIXINGS BOTH TOP AND BOTTOM HAVE BEEN INSTALLED. THE PRECAST PANELS HAVE BEEN DESIGNED FOR IN SERVICE CONDITIONS ONLY (i.e. LOADS THE PRECAST PANELS ARE SUBJECTED TO AFTER ERECTION ON SITE). THE PRECAST MANUFACTURER IS TO PROVIDE COMPUTATIONS TO THE CONTRACTOR FOR REVIEW OF STRESSES INDUCED DURING REMOVAL FROM THE MOULD, HANDLING, LIFTING, TRANSPORTATION AND ERECTION. ADDITIONAL REINFORCEMENT REQUIRED FOR THESE CONDITIONS IS THE RESPONSIBILITY OF THE PRECAST MANUFACTURER AND SHALL BE ALLOWED FOR IN THE TENDER.

PC19. GROUTED JOINTS
GROUT TO BE USED SHALL BE NON-SHRINK AND HAVE A 28 DAY CHARACTERISTIC STRENGTH OF 60MPa. PACKERS SHALL BE NON FERROUS (NEOPRENE OR EQUIV.) AND SHALL BE PROVIDED WITH 50mm MINIMUM COVER EXTERNALLY OR WHERE IN VIEW. DURING JOINT GROUTING ALL GAPS BETWEEN PRECAST ELEMENTS SHALL BE PROPERLY SEALED TO STOP THE GROUT FILLING THE REQUIRED GAPS BETWEEN THE PRECAST AND THE STRUCTURE AND TO AVOID SPOILING THE FACE OF THE PRECAST.

PC20. PANEL TYPES
PANELS CALLED UP AS SIMILAR ARE ONLY STRUCTURALLY SIMILAR, ARCHITECTURAL DIFFERENCES SUCH AS DIMENSIONS, GROOVES, REBATES, FERRULES, MINOR PENETRATIONS, ETC, MAY EXIST BETWEEN PANELS WHICH ARE STRUCTURALLY SIMILAR. REFER ARCHITECT'S DRAWINGS FOR DETAILS.

PC21. TOLERANCE
MAXIMUM TOLERANCE ON PRECAST ELEMENTS MEASURED AFTER ERECTION ON SITE TO BE +/- 20mm IN ABSOLUTE POSITION FOR ANY POINT ON THE ELEMENT AND A MAXIMUM OF +/- 5mm RELATIVE TO ANY ADJACENT PRECAST ELEMENT.

PC22. CONCRETE THICKNESS
THE MINIMUM ELEMENT STRUCTURAL THICKNESS SHALL BE 150mm OR AS NOTED ON DRAWINGS. THE DIMENSIONS ARE THE FINAL STRUCTURAL SIZES. ADDITIONAL CONCRETE THICKNESS SHALL BE PROVIDED TO ACCOMMODATE THE ARCHITECTURAL REBATES AND SURFACE TREATMENTS.

PC23. CONCRETE PROPERTIES
THE MINIMUM CONCRETE GRADES SHALL BE S40 UNLESS NOTED OTHERWISE. CONCRETE SHALL BE NORMAL WEIGHT WITH MINIMUM CEMENT CONTENT OF 330 kg/m3, S40 GRADE WILL MEET THE FOLLOWING CRITERIA:
(a) SHRINKAGE LIMIT 650 MICROSTRAIN AFTER 56 DAYS.
(b) CHLORIDE CONTENT TO BE RESTRICTED AS PER CLAUSE 4.9 OF AS 3600.
(c) NO SALT SHALL BE ADDED WITHOUT ENGINEER APPROVAL.
THE PRECAST CONTRACTOR SHALL SUBMIT THE PROPOSED MIX DESIGN FOR REVIEW BY THE ENGINEER.

PC24. REINFORCEMENT
THE REINFORCEMENT IN THE PRECAST ELEMENTS SHALL BE AS DETAILED. FABRIC SHALL BE ONE SHEET WHERE POSSIBLE. IF LAPPING IS REQUIRED IT SHALL BE TO AS 3600 CLAUSE 13.2.4. THE SPLICES SHALL NOT OCCUR WITHIN 2000mm OF THE MID SPAN OF THE PRECAST ELEMENT. WHERE TWO LAYERS OF MESH ARE USED, THE SPLICES SHALL BE STAGGERED BY 1000 mm UNLESS OTHERWISE APPROVED BY THE ENGINEER.

PC25. CONSTRUCTION JOINTS
CONSTRUCTION JOINTS IN PRECAST ELEMENTS ARE NOT PERMITTED.

PC26. GROUT REQUIREMENTS
GROUTING OF DOWELS AND JOINTS SHALL BE DONE WITH A WATERPROOF PREPACKED CEMENT-BASED PRODUCT, WHICH SHALL BE MIXED WITH WATER ON SITE TO THE RECOMMENDED CONSISTENCY. THE COMPRESSIVE STRENGTH OF GROUT MUST EXCEED 40 MPa AT 7 DAYS AND 60 MPa AT 28 DAYS, OR THE CONCRETE STRENGTH OF THE PANEL. THE PRODUCT SHALL BE STORED, HANDLED AND PLACED STRICTLY IN ACCORDANCE WITH THE MANUFACTURERS INSTRUCTIONS AND RECOMMENDATIONS. WHERE THE PRECAST ELEMENTS CARRY MORE THAN SEVEN STORIES, MAKE A PROTOTYPE OF THE JOINT USING THE PROPOSED METHOD OF CONSTRUCTION, SPLIT THE JOINT AFTER THREE DAYS FOR INSPECTION. THE DOWEL BAR NEED NOT BE USED.

PC27. INSPECTION RECORDS
THE CONTRACTOR SHALL SUBMIT DETAILED INSPECTION RECORDS FOR ALL PRECAST CONCRETE ELEMENTS.

PC28. SITE WELDING
WHERE SITE WELDING OF METAL FIXINGS IS REQUIRED IN EXPOSED AREAS, DEGREASE THE WELDED AREAS, CHIP OR GRIND OFF THE WELD SPATTER, GRIND THE WELD AREA DOWN AND COAT WITH AN APPROVED ZINC RICH PAINT TO THE MANUFACTURE'S RECOMMENDATIONS.

PC29. NON LOAD BEARING ELEMENTS
CLIP ON OR NON LOAD BEARING PRECAST ELEMENTS SHALL NOT BE ERECTED UNTIL ALL PROPS ARE REMOVED FROM THE SUPPORTING FLOOR STRUCTURE.

PC30. PENETRATIONS
NO HOLES SHALL BE CORED AND NO INSERTS SHALL BE "SHOT" OR DRILLED INTO THE PRECAST ELEMENTS

DRAWING SCHEDULE

SHEET	DRAWING No.	DRAWING TITLE
1	S01	COVER PAGE
2	S02	SITE PLAN
3	S03	FOUNDATION PLAN
4	S04	GROUND FLOOR FRAMING PLAN
5	S05	MEZZANINE FRAMING PLAN
6	S06	ROOF FRAMING PLAN
7	S07	WALL PANEL ELEVATIONS
8	S08	TYPICAL ELEVATIONS
9	S09	CONNECTION DETAILS - SHEET 1
10	S10	CONNECTION DETAILS - SHEET 2
11	S11	TYPICAL DETAILS

FOOTING SCHEDULE

REF	DESCRIPTION
SF1	500 HIGH x 600 WIDE CONCRETE GRADE: N25 REINFORCEMENT: REFER DETAIL.
SF2	500 HIGH x 600 WIDE CONCRETE GRADE: N25 REINFORCEMENT: REFER DETAIL.
F1	1600 LENGTH x1600 WIDE x 300 DEEP CONCRETE GRADE: N25 REINFORCEMENT: REFER DETAIL
F2	1600 LENGTH x1600 WIDE x 300 DEEP CONCRETE GRADE: N25 REINFORCEMENT: REFER DETAIL

NOTES:
1. GROUND BEAMS TO BE FOUNDED 100 MIN. INTO NATURAL SOIL WITH AN ALLOWABLE BEARING CAPACITY OF 100 kPa.
2. PROVIDE 15 MPa BLINDING CONCRETE AS REQUIRED UNDER FOOTINGS TO REACH FORMATION LEVEL.

FRAMING MEMBER SCHEDULE (Gr 350)

MARK	SIZE	DESCRIPTION
C1	310 UC 97 GR. 350	STEEL COLUMN
C2	150 UC 23.4 GR. 350	STEEL COLUMN
C3	90 SHS	STEEL COLUMN
SB1	250 PFC	STAIR BEAM
B1	310 UC 97 GR. 350	STEEL BEAM
B2	310 UB 46.2 GR. 350	COMPOSITE BEAM
WB1	180 PFC GR. 350	WIND BEAM
ST1	100x100x5 SHS GR. 350	STRUT BEAM
ST2	100x100x5 SHS GR. 350	STRUT BEAM
RP1	C20024@1200mm	ROOF PURLINS

GROUND BEARING SLAB SCHEDULE

REF	DESCRIPTION
SOG1	100 THK SLAB ON GROUND CONCRETE GRADE: N25 REINFORCEMENT: SL92 MESH TOP (25 COVER)
SOG2	150 THK SLAB ON GROUND CONCRETE GRADE: N25 REINFORCEMENT: SL92 MESH TOP (25 COVER)

SLAB IS TO BE CAST ON 0.2 mm THK MOISTURE BARRIER ON NATURAL GROUND OF 100 kPa BEARING PRESSURE OR ON CONTROLLED COMPACTED FILL.

ALL EARTH FILLING IS TO BE CLEAN AND NON-EXPANSIVE SOIL PLACED AND COMPACTED TO AS2870-2011 CLAUSE 6.4.2.

N

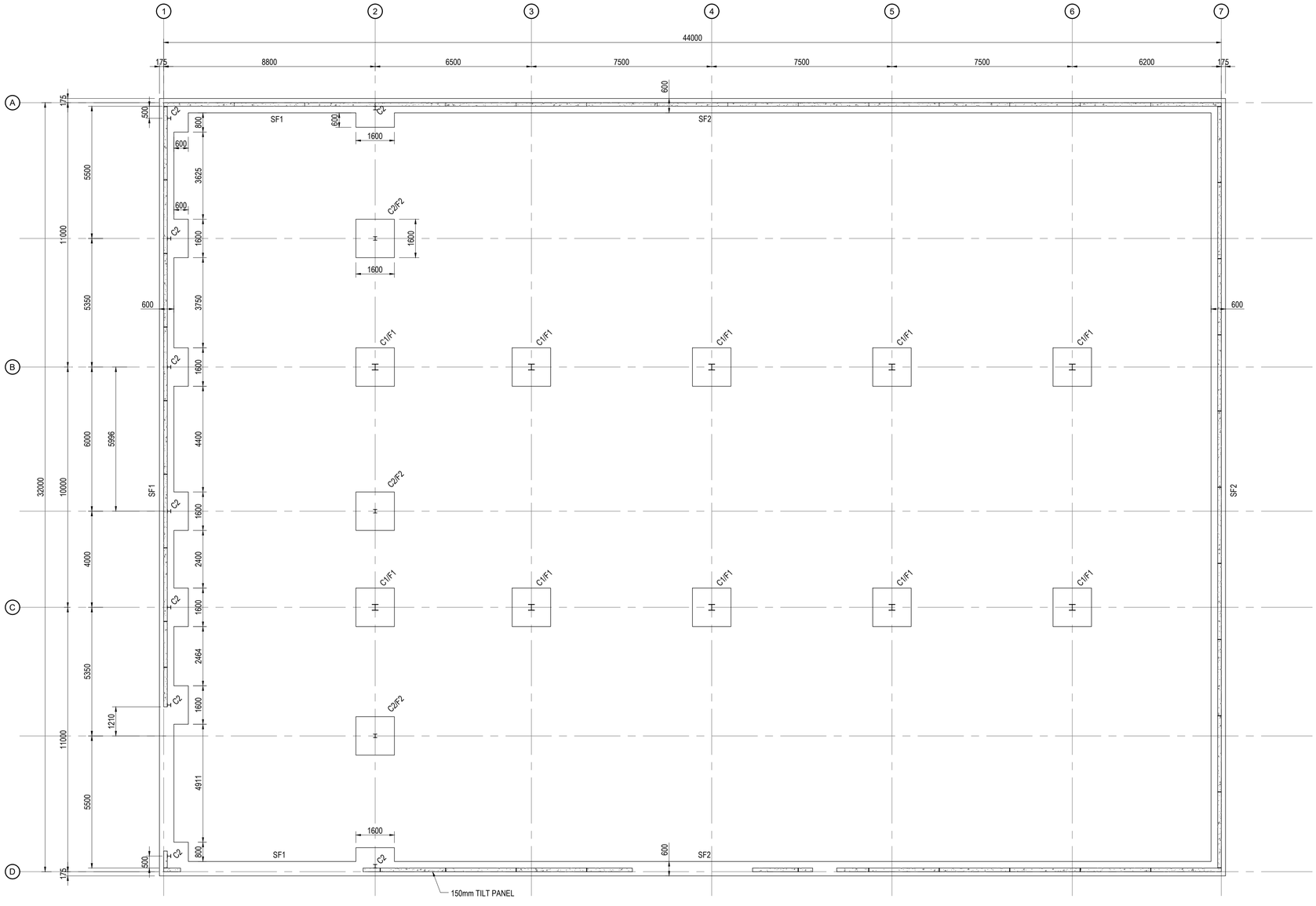
ROXIMATE LOCATION OF WORKS

LOCALITY PLAN
N.T.S.

PUBLIC PORTFOLIO SAMPLE ? CLIENT, SITE, COMPANY AND PROJECT IDENTIFIERS REDACTED

[redacted] [redacted] [redacted] [redacted]

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FOUNDATION PLAN

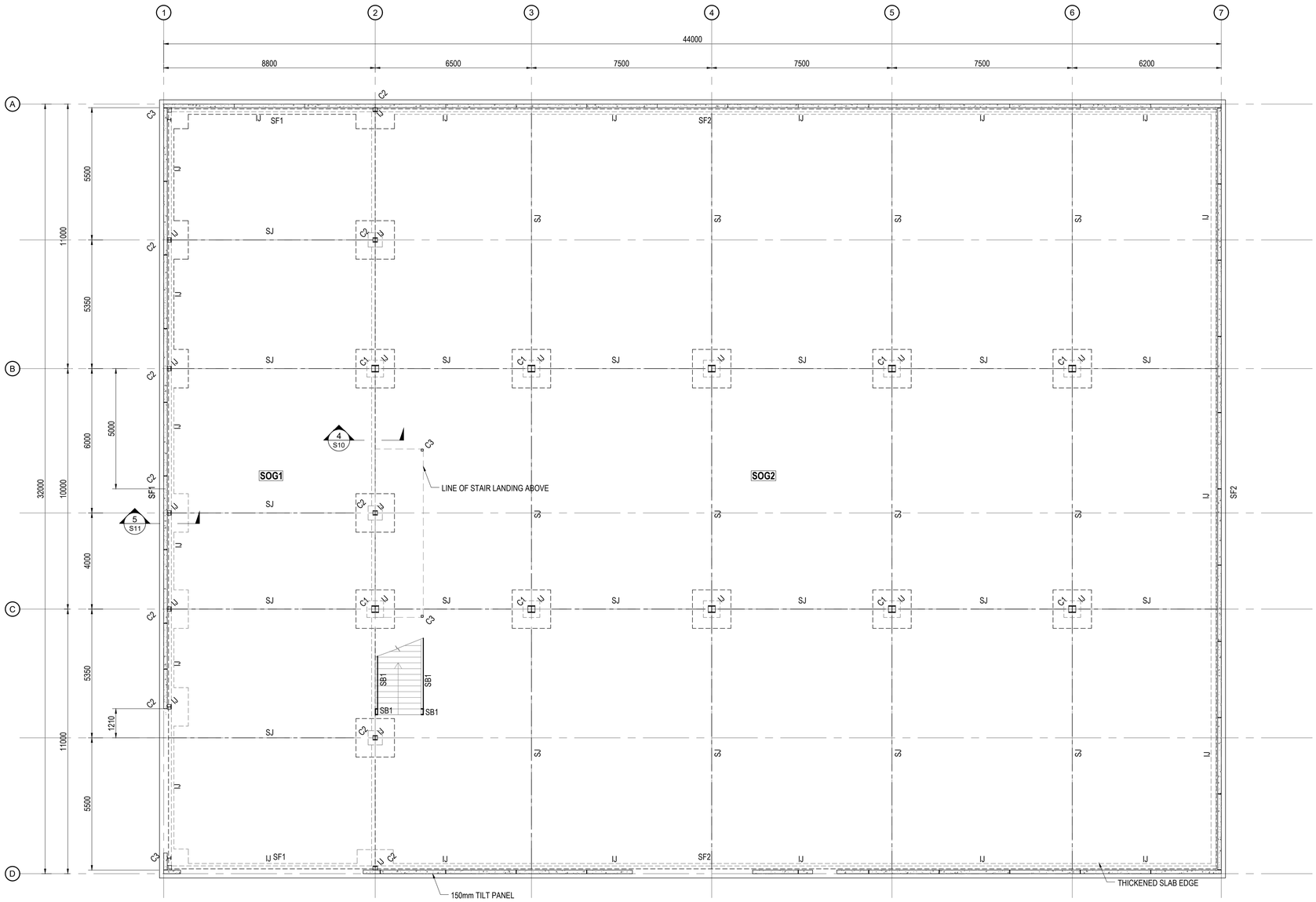
SCALE 1:100

NOTE:

-FOR MEMBER SCHEDULE, REFER TO SHEET S01.

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GROUND FLOOR FRAMING PLAN

SCALE 1:100

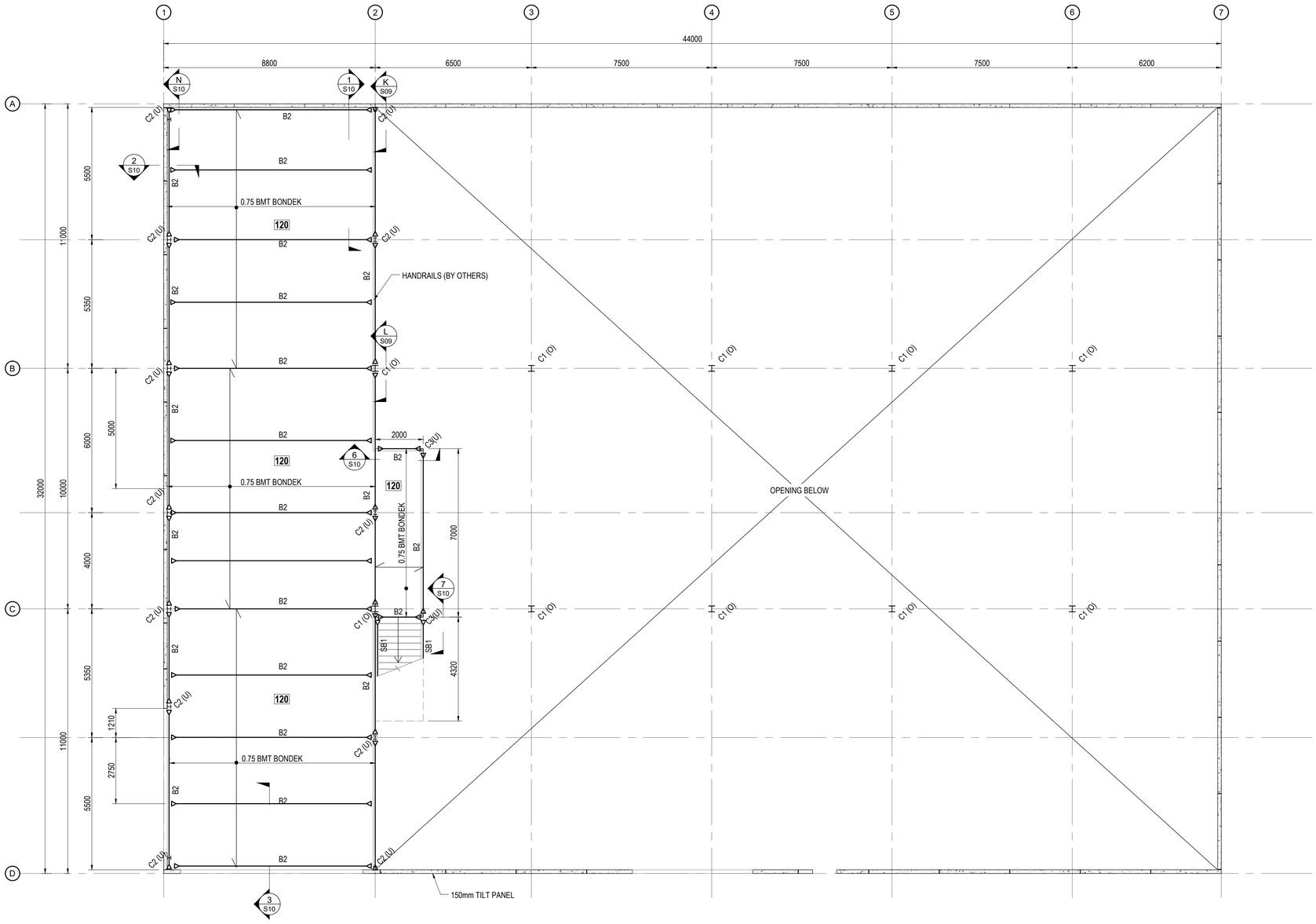
NOTE:

-FOR MEMBER SCHEDULE, REFER TO SHEET S01.

IJ - DENOTES ISOLATION JOINT
SJ - DENOTES SAWCUT JOINT

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MEZZANINE FRAMING PLAN

SCALE 1:100

LEGENDS:

- FIXED CONNECTION
- PINNED CONNECTION
- BEAM OVER

NOTE:

-FOR MEMBER SCHEDULE, REFER TO SHEET S01.

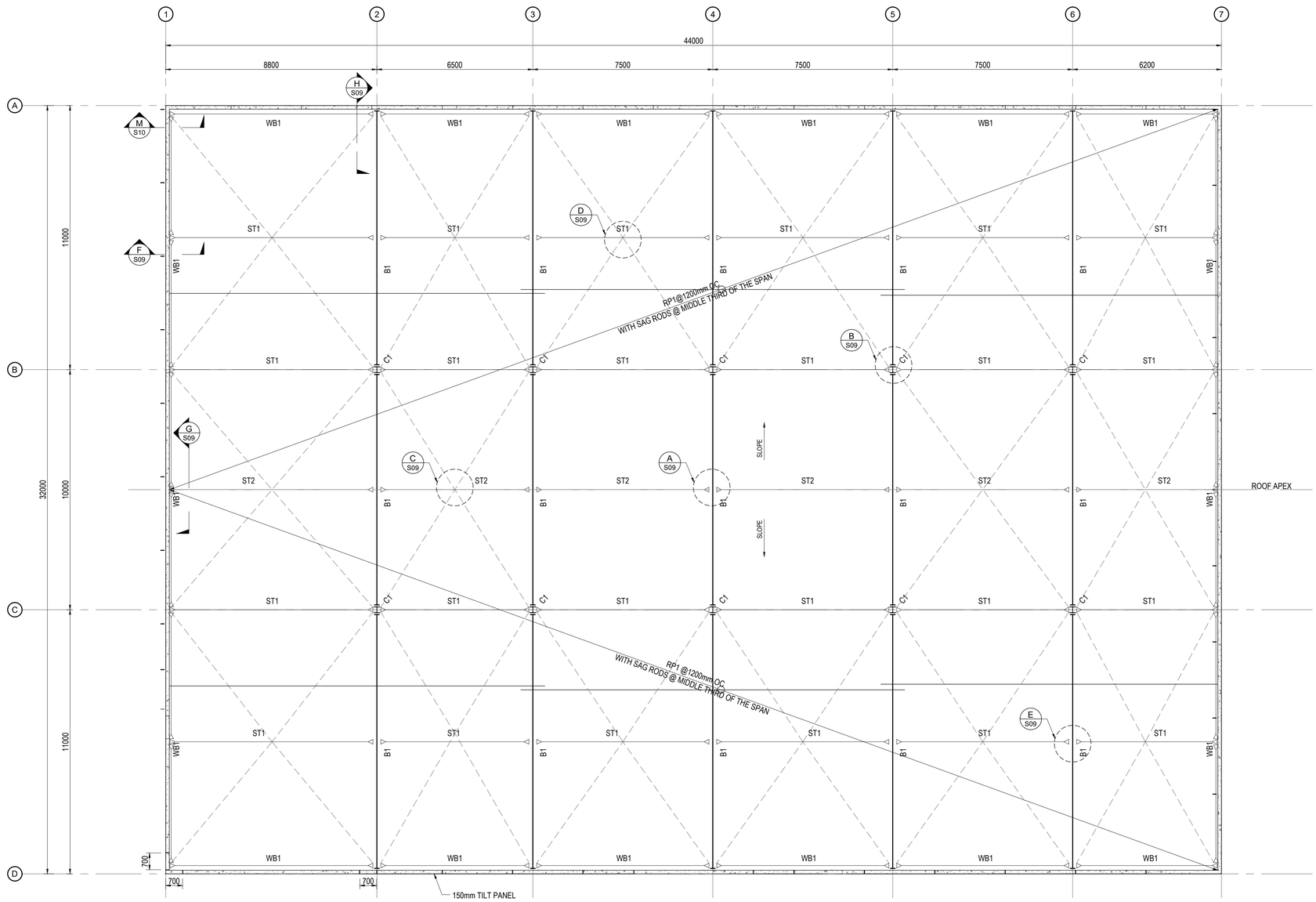
-CONSTRUCTION STAGE ANALYSIS SHALL BE PERFORMED BY OTHERS TO IDENTIFY THE FOLLOWING:

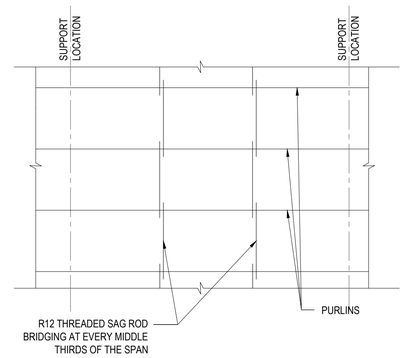
1. MEZZANINE PROPPING SUPPORT DURING CONCRETE POURING.
2. TILT UP PANEL LIFTING ANCHOR POINTS.
3. TILT UP PANEL TEMPORARY BRACING DURING ERECTION.

(U) - DENOTES COLUMN UNDER
(O) - DENOTES COLUMN OVER

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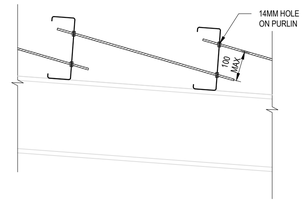
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SAGROD LAYOUT

NTS



TYP SAGROD DETAIL

NTS

ROOF FRAMING PLAN

SCALE 1:100

LEGENDS:

FIXED CONNECTION

PINNED CONNECTION

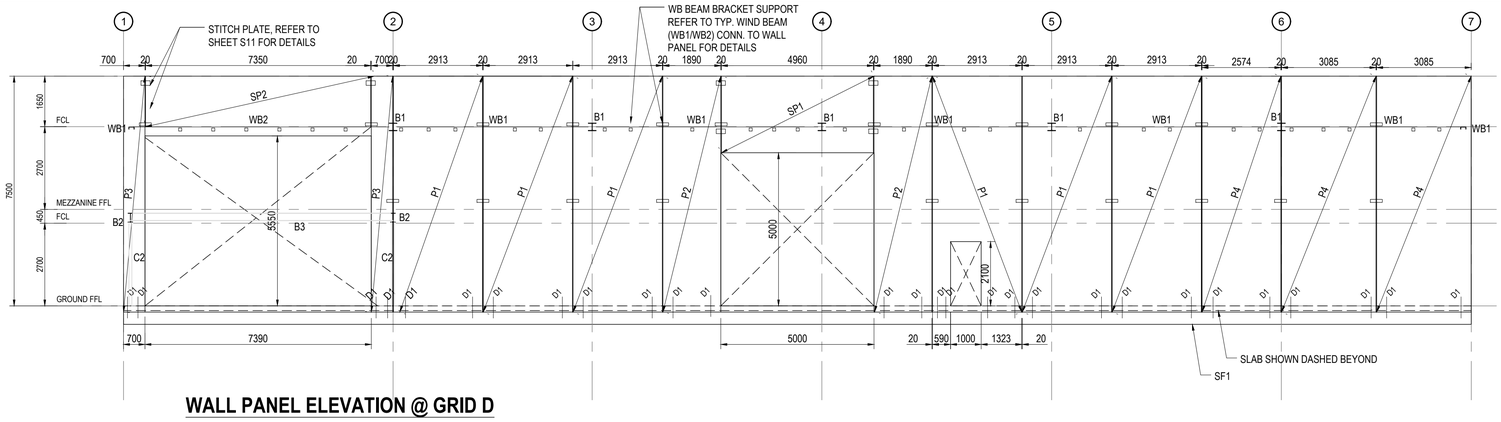
16mm REID BRACE

NOTE:

-FOR MEMBER SCHEDULE, REFER TO SHEET S01.

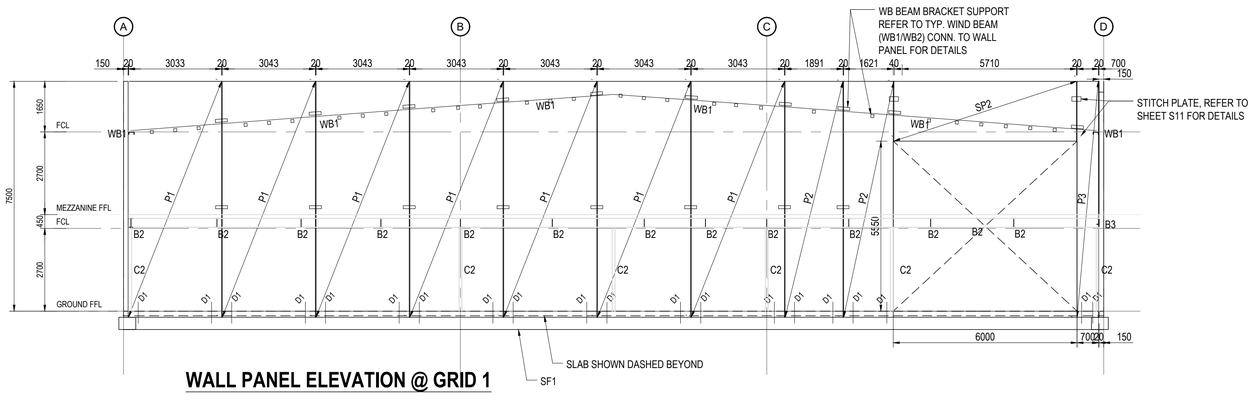
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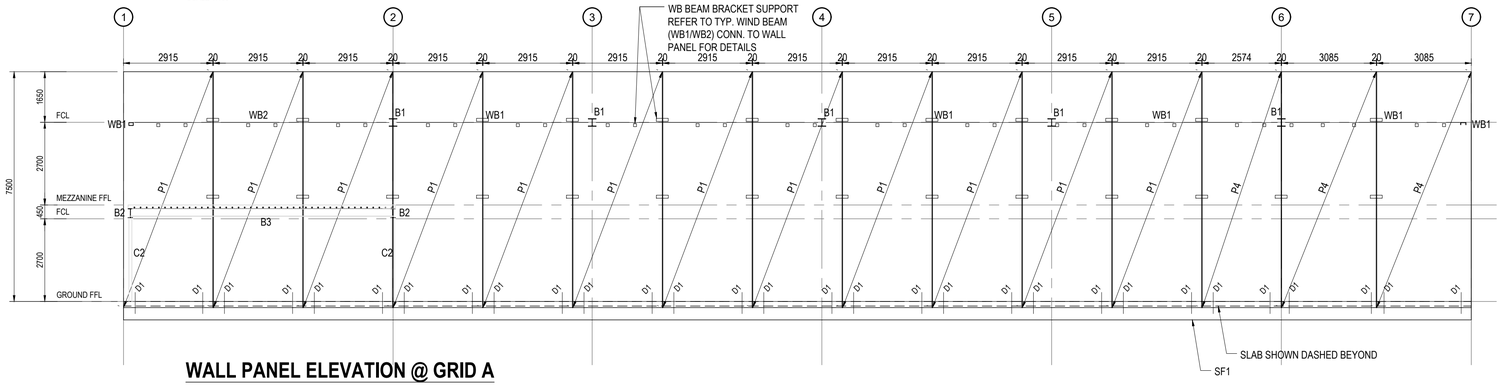
WALL PANEL ELEVATION @ GRID D

SCALE 1:100



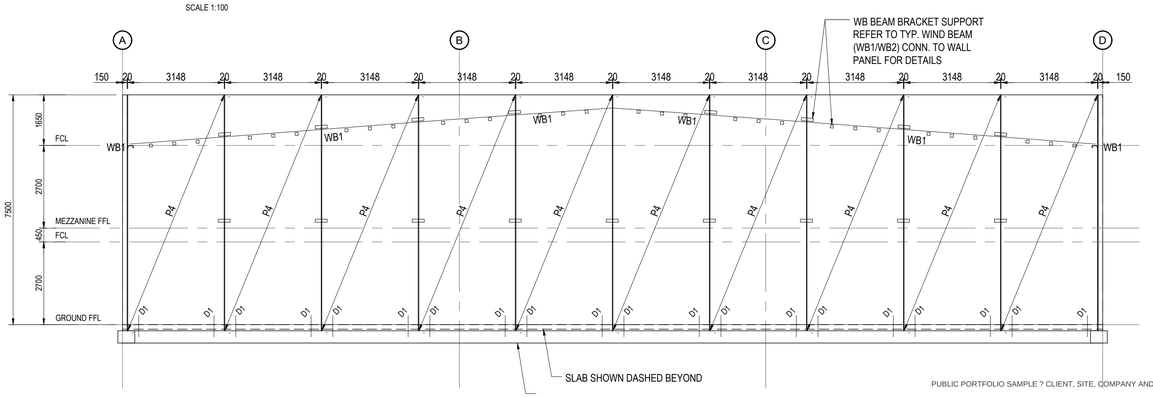
WALL PANEL ELEVATION @ GRID 1

SCALE 1:100

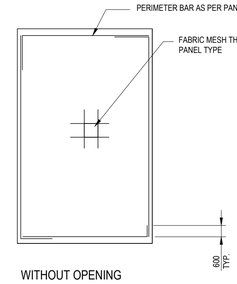


WALL PANEL ELEVATION @ GRID A

SCALE 1:100



PRECAST WALL SCHEDULE		
MARK	THICKNESS	REINFORCEMENT
P1	150mm	RL1118 MESH CENTRAL, 2N16 PERIMETER BARS
P2	150mm	RL918 MESH CENTRAL, 2N16 PERIMETER BARS
P3	150mm	150 / 700 / 5N16- VERT AND CENTRAL / N10-200 HOR.
P4	150mm	RL818 MESH CENTRAL, 2N16 PERIMETER BARS
SP1	150mm	VARIES / 4960 MAX / N12@200 / 150 / 2500 / 2000 / N10@200
SP2	150mm	VARIES / 7350 MAX / N12@200 / 150 / 1960 / 1650 / N10@200



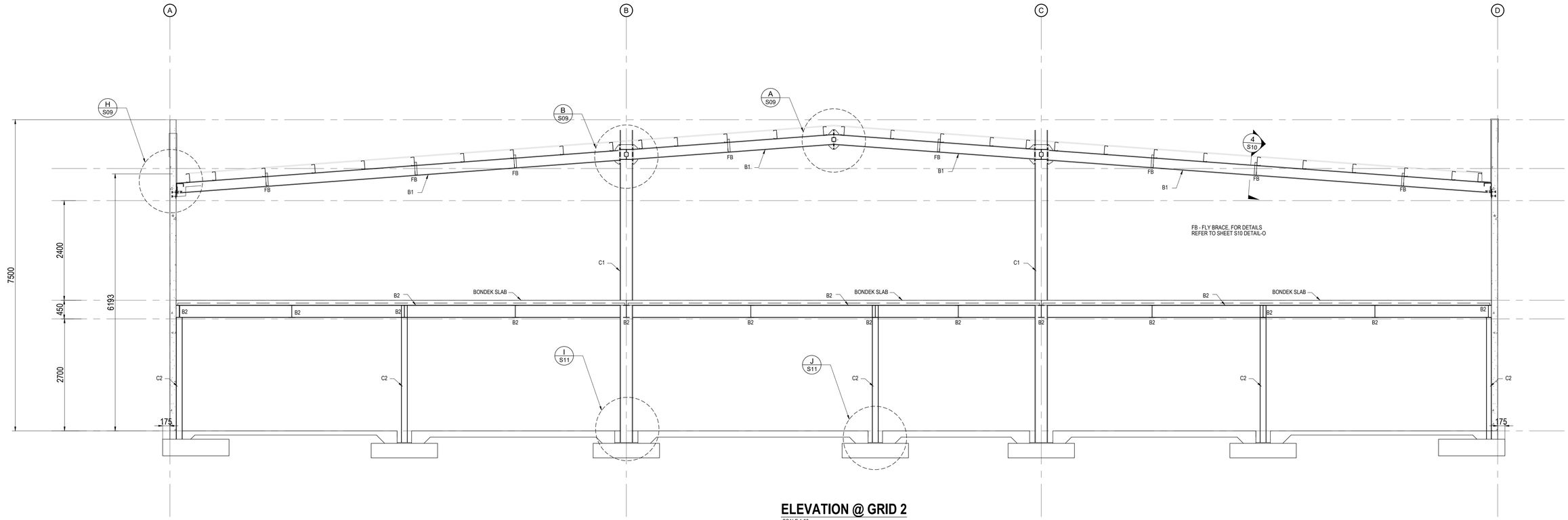
NOTES:
1- REFER TO WALL ELEVATIONS FOR TYPICAL REINFORCEMENT, CAST-IN PLATES, DOWELS AND STITCH PLATE LOCATIONS
2- PROVIDE PROPPING AND TEMPORARY WORKS IN ACCORDANCE WITH TEMPORARY WORKS ENGINEERS DESIGN AND DRAWINGS
3- REFER TO TYPICAL DETAILS FOR ADDITIONAL REINFORCEMENT AT CAST-IN ITEMS

TYPICAL PANEL

NTS

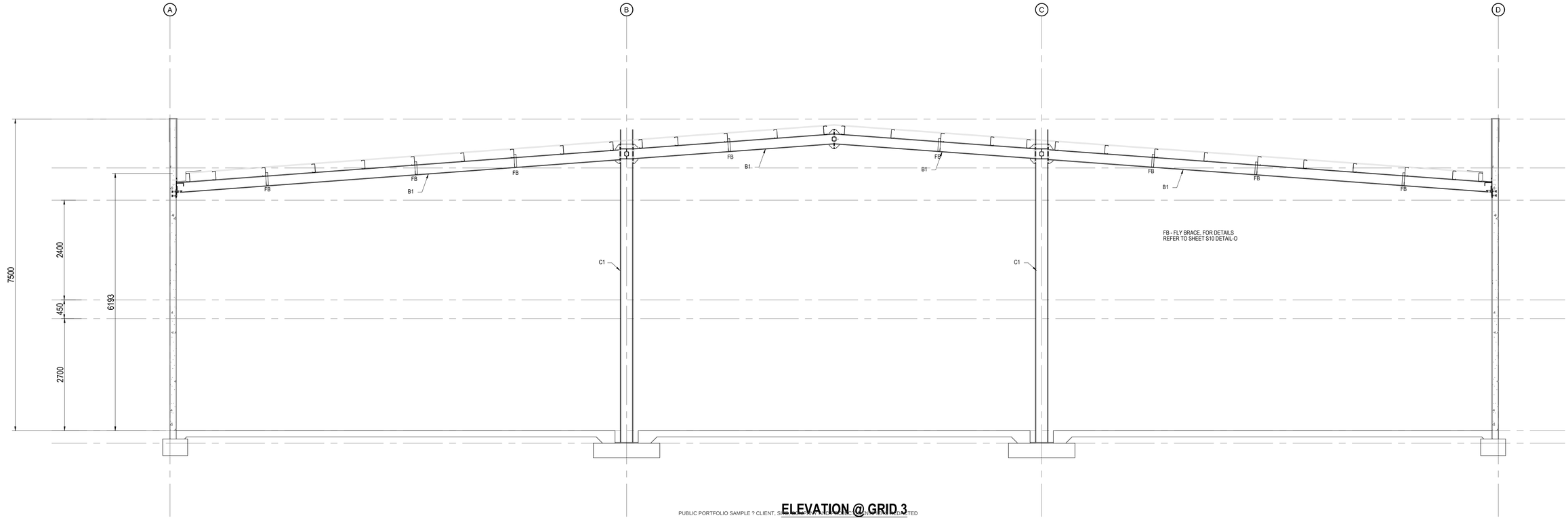
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ELEVATION @ GRID 2

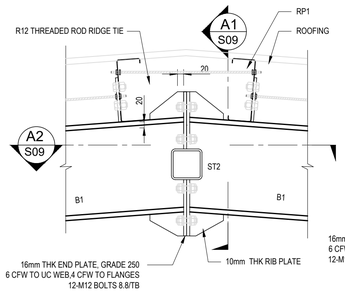
SCALE 1:50



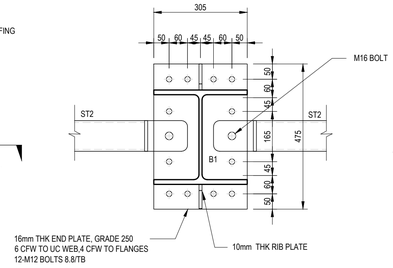
ELEVATION @ GRID 3

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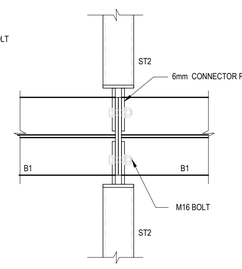
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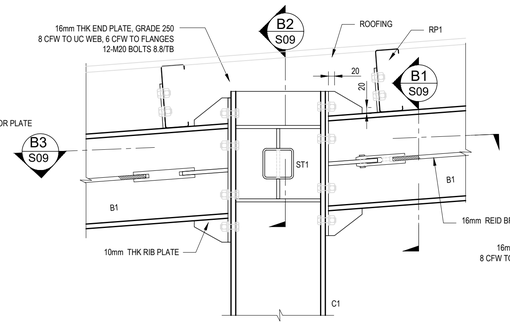
DETAIL A
SCALE 1:10 — A / S08



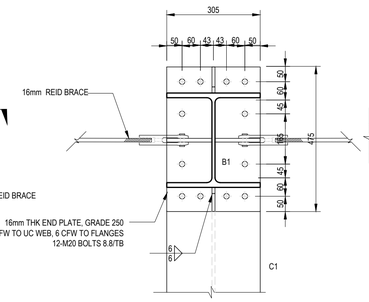
SECTION
SCALE 1:10 — A1 / S09



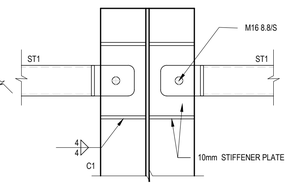
SECTION
SCALE 1:10 — A2 / S09



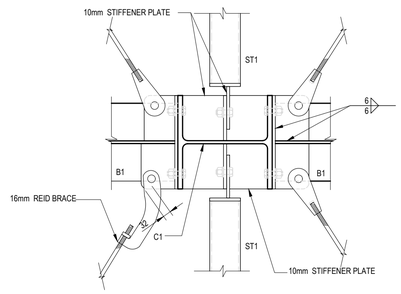
DETAIL B
SCALE 1:10 — B / S08



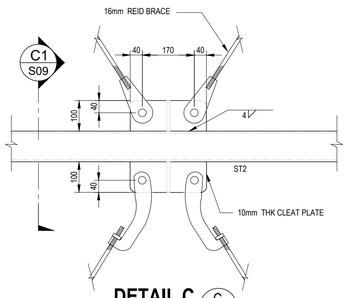
SECTION
SCALE 1:10 — B1 / S09



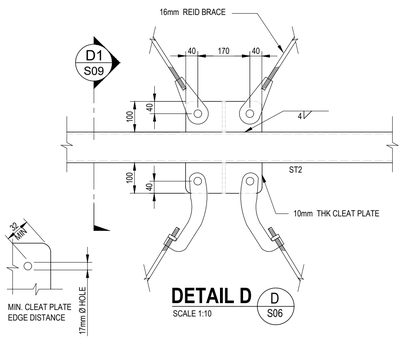
SECTION
SCALE 1:10 — B2 / S09



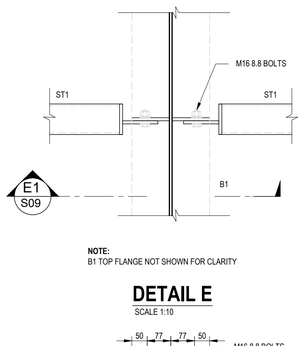
SECTION
SCALE 1:10 — B3 / S09



DETAIL C
SCALE 1:10 — C / S06

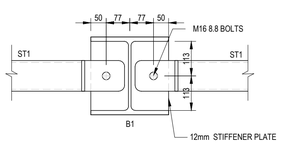


DETAIL D
SCALE 1:10 — D / S06

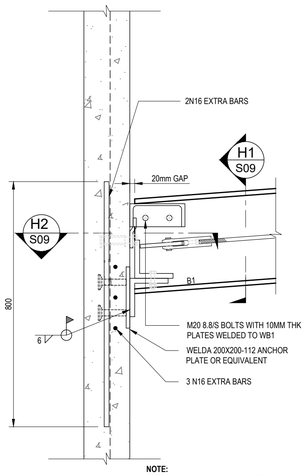


NOTE:
B1 TOP FLANGE NOT SHOWN FOR CLARITY

DETAIL E
SCALE 1:10

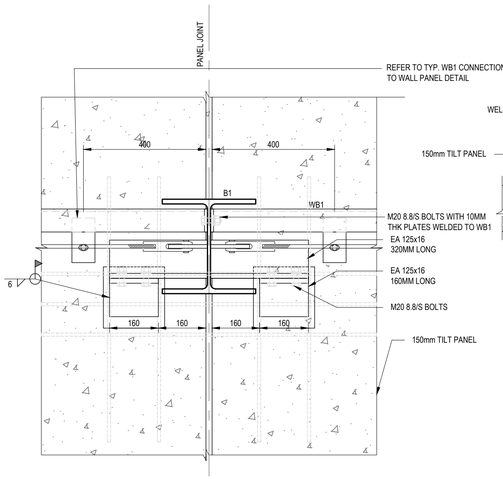


SECTION
SCALE 1:10 — E1 / S09

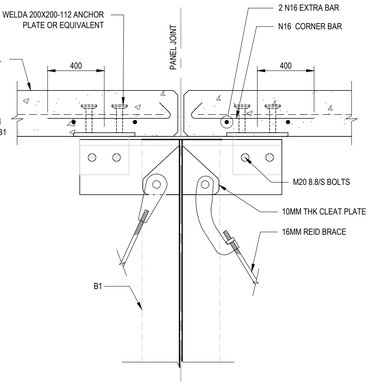


NOTE:
PURLINS NOT SHOWN FOR CLARITY

DETAIL H
SCALE 1:10 — H / S08

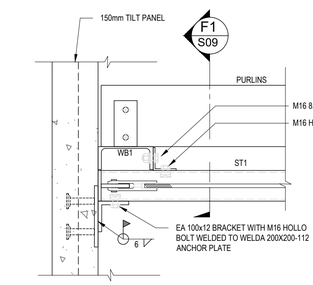


SECTION
SCALE 1:10 — H1 / S09

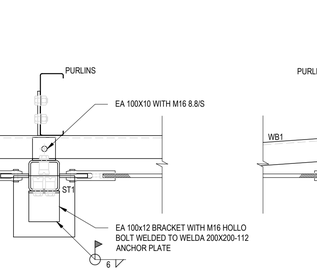


NOTE:
B1 TOP FLANGE NOT SHOWN FOR CLARITY

SECTION
SCALE 1:10 — H2 / S09



SECTION
SCALE 1:10 — C1 / S09

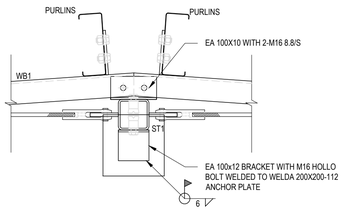


SECTION
SCALE 1:10 — D1 / S09

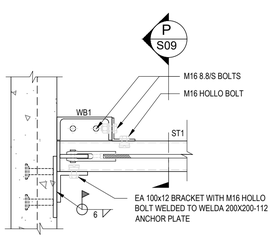
DETAIL F
SCALE 1:10 — F / S06

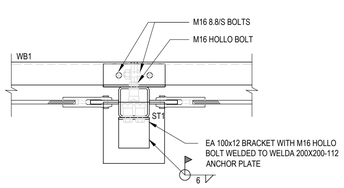
NOTE:
FOR WB1 TO WALL PANEL CONNECTION,
REFER TO TYPICAL WIND BEAM TO WALL
PANEL CONNECTION DETAILS ON ON SHEET S11.

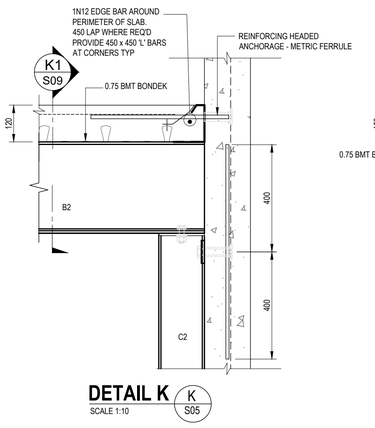
SECTION
SCALE 1:10 — F1 / S09



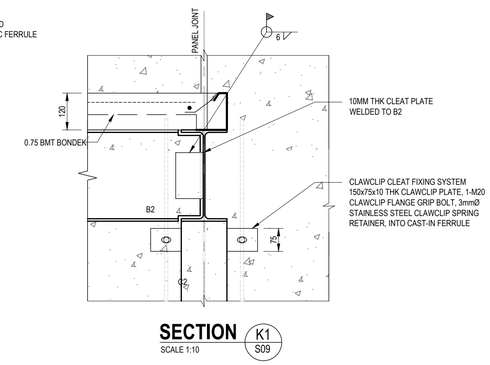
DETAIL G
SCALE 1:10 — G / S06



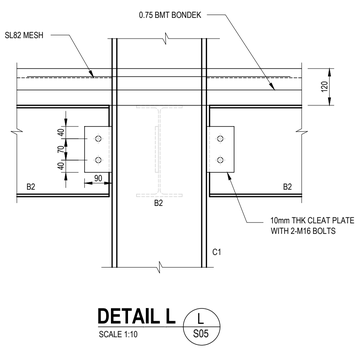




DETAIL K
SCALE 1:10 — K / S05



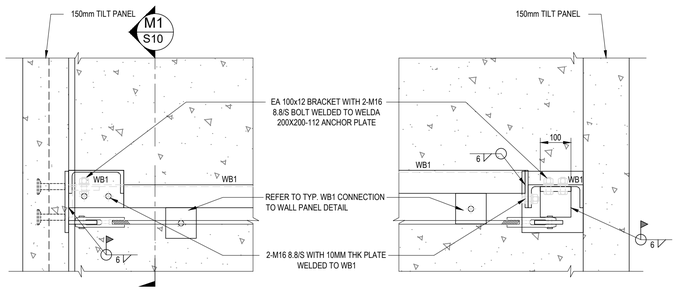
SECTION
SCALE 1:10 — K1 / S09



DETAIL L
SCALE 1:10 — L / S05

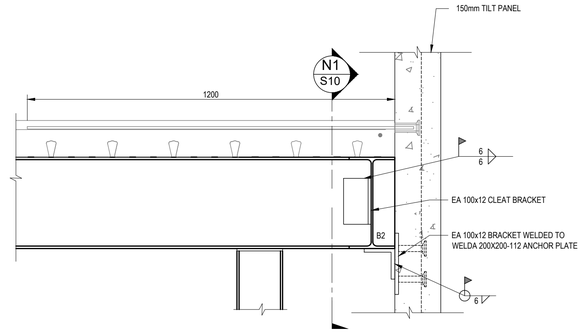
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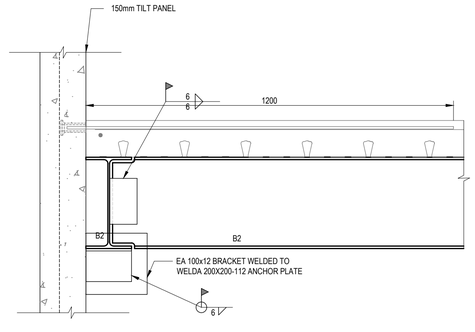


DETAIL M
SCALE 1:10 — M / S06

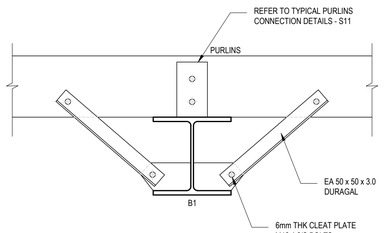
SECTION
SCALE 1:10 — M1 / S10



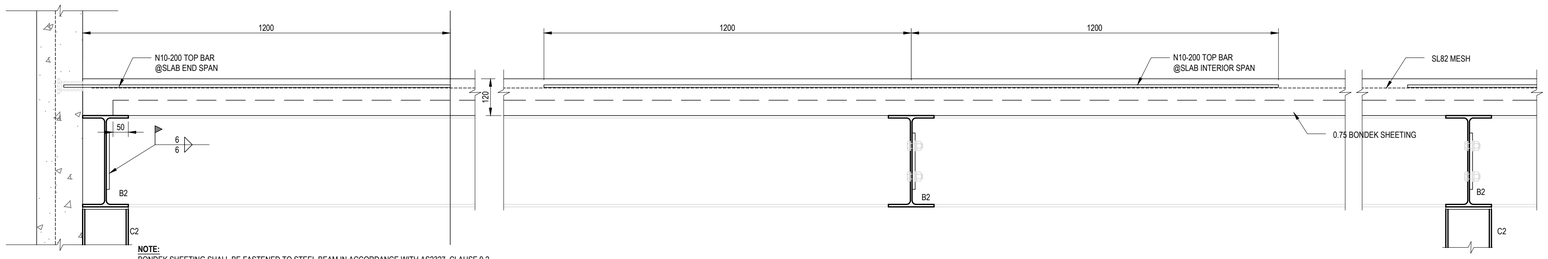
SECTION
SCALE 1:10 — N / S05



SECTION
SCALE 1:10 — N1 / S10

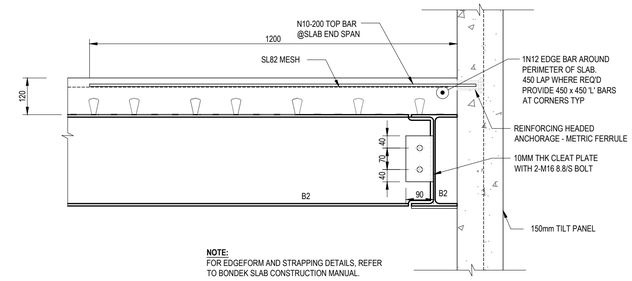


SECTION
SCALE 1:10 — O / S08



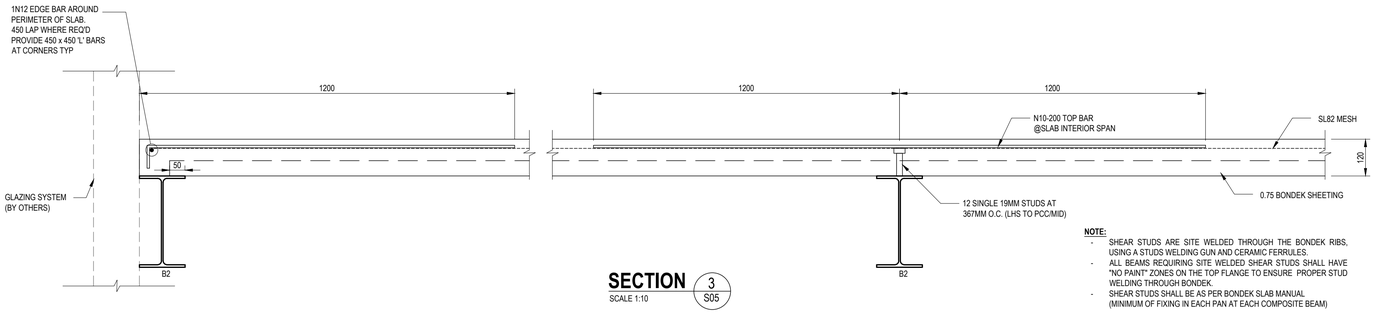
NOTE:
BONDEK SHEETING SHALL BE FASTENED TO STEEL BEAM IN ACCORDANCE WITH AS2327, CLAUSE 9.2.

SECTION
SCALE 1:10 — 1 / S05



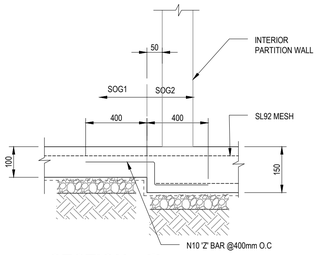
NOTE:
FOR EDGEFORM AND STRAPPING DETAILS, REFER TO BONDEK SLAB CONSTRUCTION MANUAL.

SECTION
SCALE 1:10 — 2 / S05

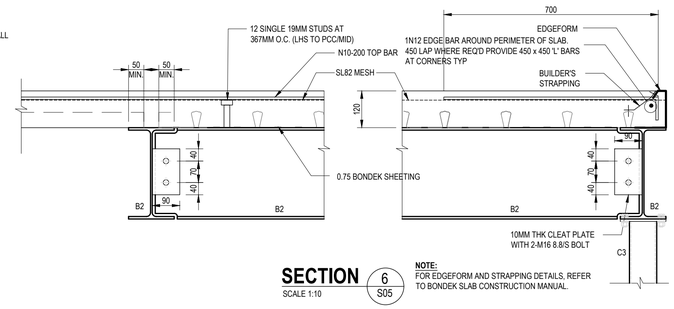


NOTE:
- SHEAR STUDS ARE SITE WELDED THROUGH THE BONDEK RIBS, USING A STUDS WELDING GUN AND CERAMIC FERRULES.
- ALL BEAMS REQUIRING SITE WELDED SHEAR STUDS SHALL HAVE "NO PAINT" ZONES ON THE TOP FLANGE TO ENSURE PROPER STUD WELDING THROUGH BONDEK.
- SHEAR STUDS SHALL BE AS PER BONDEK SLAB MANUAL (MINIMUM OF FIXING IN EACH PAN AT EACH COMPOSITE BEAM)

SECTION
SCALE 1:10 — 3 / S05

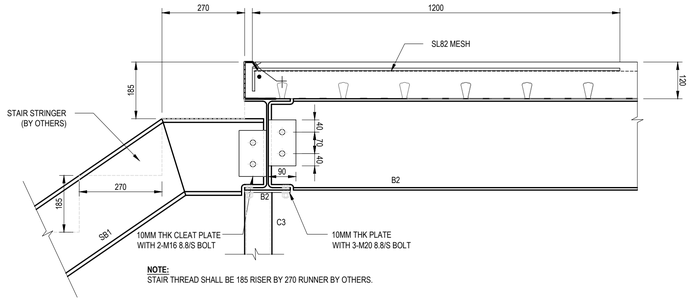


SECTION
SCALE 1:10 — 4 / S04



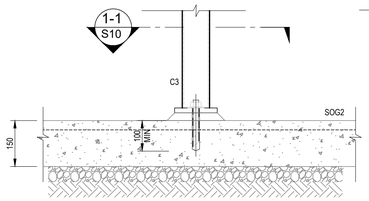
NOTE:
FOR EDGEFORM AND STRAPPING DETAILS, REFER TO BONDEK SLAB CONSTRUCTION MANUAL.

SECTION
SCALE 1:10 — 6 / S05

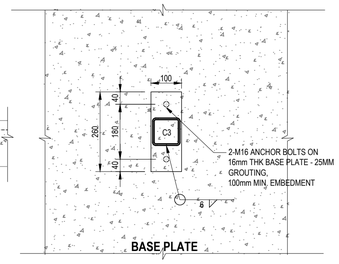


NOTE:
STAIR THREAD SHALL BE 185 RISER BY 270 RUNNER BY OTHERS.

SECTION
SCALE 1:10 — 7 / S05

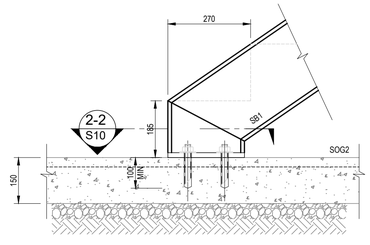


C3 FOOTING DETAIL
SCALE 1:10



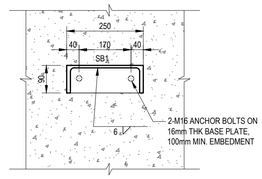
BASE PLATE

SECTION
SCALE 1:10 — 1-1 / S10



NOTE:
STAIR THREAD SHALL BE 185 RISER BY 270 RUNNER BY OTHERS.

SB1 FOOTING DETAIL
SCALE 1:10

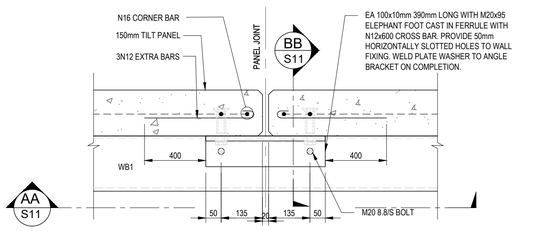


BASE PLATE

SECTION
SCALE 1:10 — 2-2 / S10

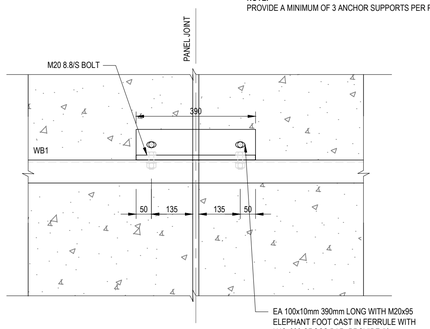
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TYPICAL WIND BEAM (WB1) CONNECTION TO WALL PANEL JOINT

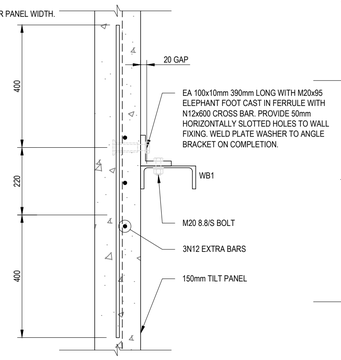
SCALE 1:10



NOTE:
PROVIDE A MINIMUM OF 3 ANCHOR SUPPORTS PER PANEL WIDTH.

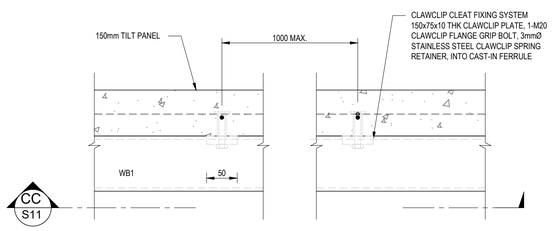
SECTION AA S11

SCALE 1:10



SECTION BB S11

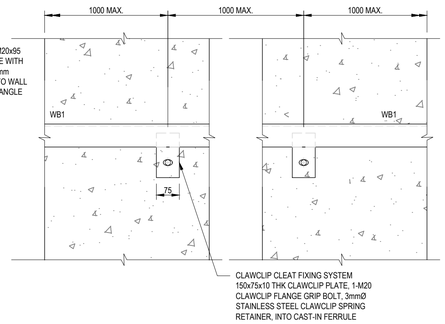
SCALE 1:10



TYPICAL WIND BEAM (WB1) CONNECTION TO WALL PANEL JOINT (INTERNAL PANEL)

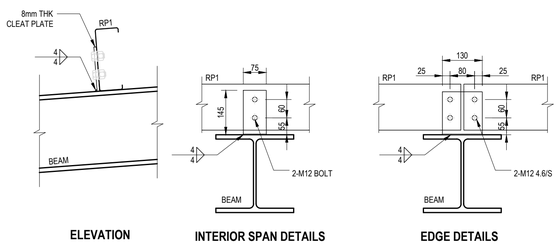
SCALE 1:10

NOTE:
PROVIDE A MINIMUM OF 3 ANCHOR SUPPORTS PER PANEL WIDTH.



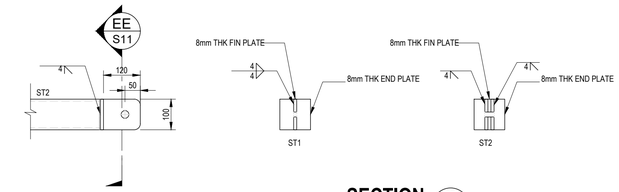
SECTION CC S11

SCALE 1:10



TYPICAL PURLINS CONNECTION DETAILS

SCALE 1:10

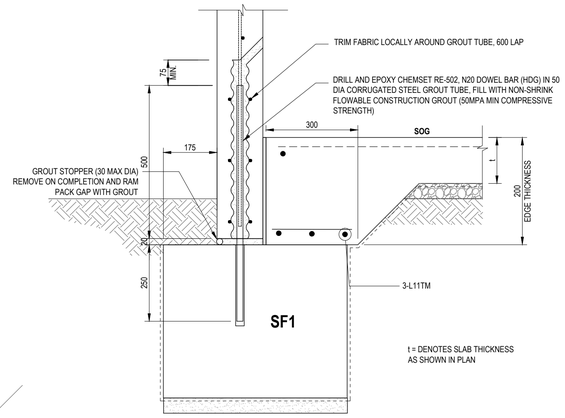


TYPICAL STRUT FIN PLATE DETAILS

SCALE 1:10

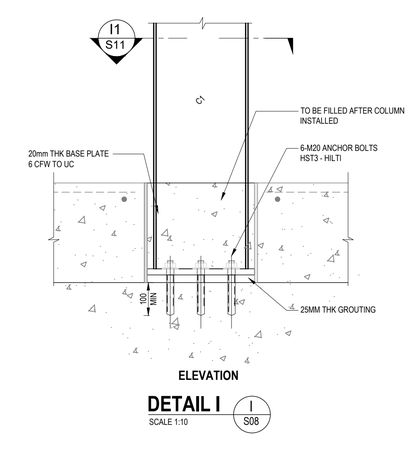
SECTION EE S11

SCALE 1:10



TYPICAL PRECAST DOWEL AT BASE TYPE

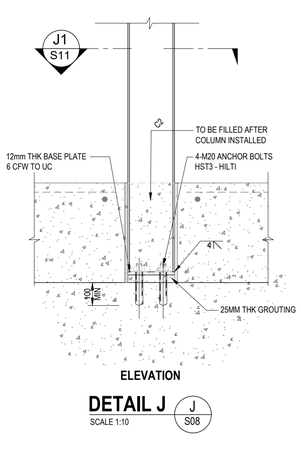
SCALE 1:10



ELEVATION

DETAIL I I S08

SCALE 1:10



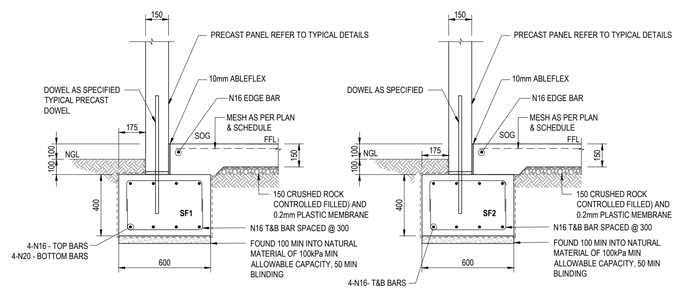
ELEVATION

DETAIL J J S08

SCALE 1:10

TYPICAL STITCH PLATE DETAIL TYPE (SP1)

SCALE 1:10

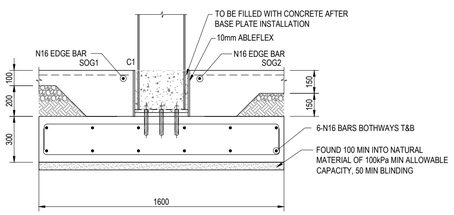


TYP. STRIP FOOTING (SF1) DETAIL

SCALE 1:20

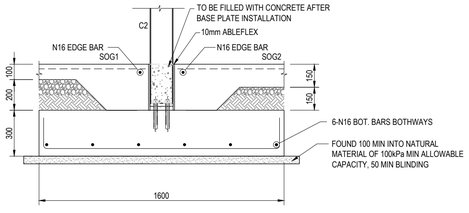
TYP. STRIP FOOTING (SF2) DETAIL

SCALE 1:20



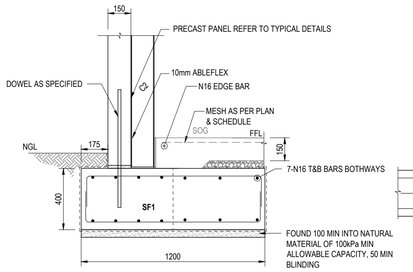
TYPICAL PAD FOOTING (F1) DETAIL

SCALE 1:20



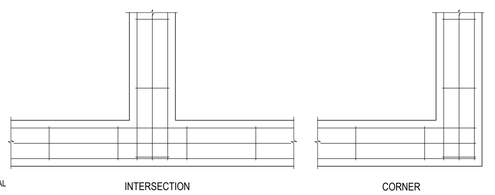
TYPICAL PAD FOOTING (F2) DETAIL

SCALE 1:20



SECTION 5 S04

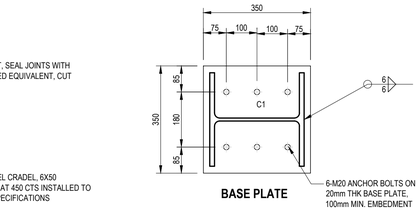
SCALE 1:20



NOTE:
LAP FULL WIDTH OF TRENCH MESH AT CORNERS AND INTERSECTIONS. LAP 500mm MIN AT SPLICES

TYPICAL PLAN VIEW - STRIP FOOTINGS

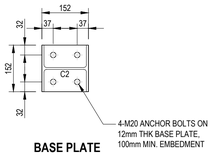
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BASE PLATE

SECTION I1 S11

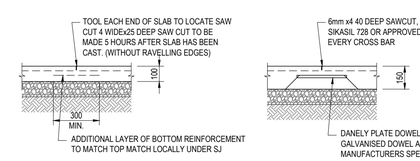
SCALE 1:10



BASE PLATE

SECTION J1 S11

SCALE 1:10



NOTE:
SAWCUT WITHIN 12HRS OF POURING SLAB.

TYPICAL SAW CUT JOINT (SJ) DETAIL

SCALE 1:20

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